

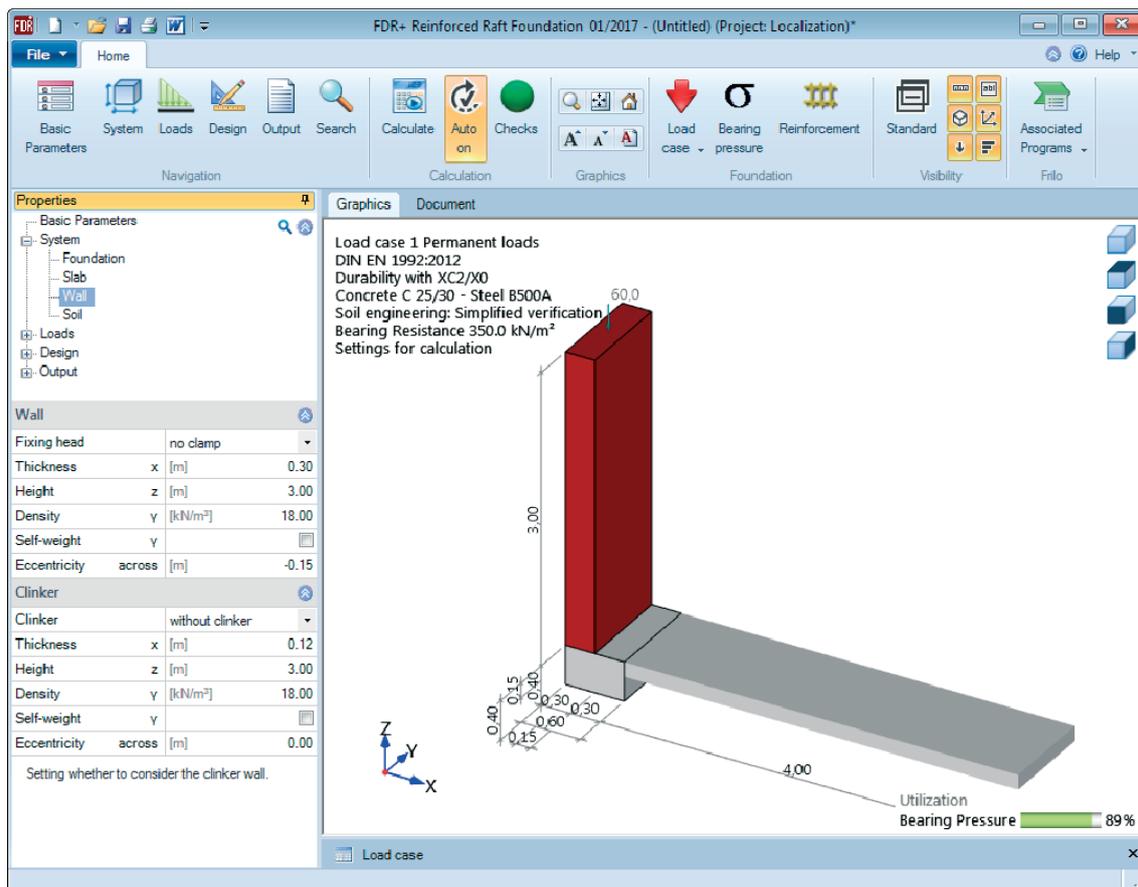
Reinforced Raft Foundation FDR+

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As of 22/02/2017



Reinforced Raft Foundation FDR+

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Application options

The FDR+ application allows the design of eccentrically loaded boundary foundations that are connected to a reinforced concrete slab with a rigid joint. In the design, the centring moment, the centring tensile force and the soil pressure are determined with consideration to deformations.

Properties

- Selection options concerning the durability requirements
- Load definition: moments, axial forces, horizontal loads
- Different load cases, that apply alternatively or simultaneously according to the user's selection, are automatically superimposed
- Determination of the base pressure as well as the design value of the base pressure resistance with the help of tables in the selected soil engineering standards or of user-defined tables taken from a soil expertise, for instance
- Examination and consideration of a gaping joint
- Consideration of the accidental design situations BS-A and BS-E
- Interface to the FRILO Building Model (GEO)
- The self-weights of the wall, the facing masonry and the foundation can be selected independently of each other
- Optional calculation of the connecting reinforcement of the rising wall to the foundation
- Bending design of the foundation and verification whether reinforcement could be dispensed with in the lower layer of the foundation
- Centring in the rigidly connected reinforced concrete slab in accordance with J. Kanya, Bautechnik 05/1969
- Simultaneous restraint in the wall and the slab is optionally selectable
- Bending design at the connection between the foundation and the reinforced concrete slab
- Calculation of settlement effects
- Calculation of the foundation's deformation
- Verification of the concrete compression stress and the steel tensile stress at the connection to the slab
- Crack width verification at the connection to the slab
- Ground failure verification with consideration of berms and the anchoring depth of the foundation
- Simplified verification using the design value of the base pressure resistance as a rule
- Reinforcing steel mesh, steel bar or user-defined A_s values are the available reinforcement options

Limits of application

The following conditions in accordance with [Kanya](#), Bautechnik 05-1969 are to be complied with when using the software:

- The foundation of the building is designed in such a way as to ensure that all foundations are subject to the same average settlement in the centre of gravity of their surface areas (no settlement variations).
- The eccentrically loaded border foundation can rotate around the fulcrum "D".
- The adjacent central foundation is torsionally stiff.
- A pure structural system is assumed, i.e. there are no disturbing connecting devices parallel to the supporting direction of the centring plate.

- The centring plate is appropriately reinforced and softly supported. No external influence acts additionally on the centring plate.
- The border foundation is infinitely stiff in itself.
- The self-weight of the centring plate is negligibly small compared to the applying load.

Note: ***The stiffness modulus should be selected with utmost care.***
Because the cross section in the connection between the floor slab and the foundation can tear off widely, you can reduce the stiffness of the floor slab with a pre-factor. You can also define a factor for the bending stiffness of the wall.

Actions and loads

Loads are always defined with characteristic values. You can define loads as acting alternatively. The alternative group numbers are available for this. When you assign the alternative group 0 to the defined load, this means that it can participate in all load combinations generated with the combination rules. If two or more loads are members of the same alternative group, they never act simultaneously.

For the structural components wall, facing masonry and foundation, you can activate or deactivate the **self-weight** separately. The activated self-weight portions, which are calculated automatically, are included in the combinatorial analysis.

Verifications in the ultimate limit state

You can select different concrete types and reinforcing steels for the wall, the foundation and the connected reinforced concrete slab. You can also define masonry for the wall. In order to provide for the required reinforcement, you can define woven steel fabric and/or rebar. If the selected reinforcement exceeds the required quantity in the ultimate limit state, it is included in the verification of the serviceability limit state instead of the required reinforcement. Dialogs for the selection of the exposure classes and the determination of the shrinkage coefficient and the creep factor are available in connection with the durability and serviceability requirements. The resulting concrete coverage and reinforcement layers are taken into account. The bending design is based on the kh (kd) method. If the wall is connected to the foundation in a deflection-resistant manner, the foundation is dimensioned in the contact face of the wall. Otherwise, the bending moment centrally underneath the wall is taken into account in the design. The minimum reinforcements of the wall, the foundation and the slab can be selected independently of each other. The software checks whether the foundation can be installed without reinforcement in the lower layer. The shear force analysis is performed at the distance from the wall that is equal to the structurally effective height d. The user can select whether the foundation should be designed as a reinforced concrete slab or a reinforced concrete beam. The shear design as a reinforced concrete beam produces the minimum shear reinforcement in each case.

Verifications in the serviceability limit state

The deformation of the foundation is calculated for the quasi permanent and infrequent load combinations. In this calculation, the displacement of individual points in the foundation is indicated as a fraction of the foundation width (e.g. L/500) and torsion is specified in degrees. In addition to the deformation analysis, verifications are performed in accordance with the selected reinforced concrete standard. They include verifications of the compressive concrete stress, the tensile steel stress as well as the calculation of the existing crack width and the limit diameter of the reinforcement at the connection of the reinforced concrete slab and the foundation. In these calculations, a creep factor that can optionally either be defined by the user or be calculated by the software is taken into account.

Verifications in the ultimate limit state

- Simplified verification, normally using the design value of the base pressure resistance

Based on the calculation method by [Kanya](#), the software calculates a trapezoidal or, if a gaping joint occurs, a triangular base pressure distribution, which is compared to the selected design value of the base pressure resistance. Optionally, the permissible base pressure can be taken from a table in the selected foundation engineering standard, a table in a soil expertise or the user can enter a user-defined value. As far as the gaping joint is concerned, the software checks whether a gaping joint occurs when only permanent loads apply and whether the gaping joint produced by permanent and variable loads is greater than half of the foundation width.

- Ground failure analysis

In addition to the verification of the base pressure, the FDR+ software offers the possibility of performing a ground failure analysis as per DIN 4017 [2006-03] or ÖNORM B 4435-2 [1999-10]. In this verification, a homogenous soil layer above the foundation base and a homogenous soil layer underneath the foundation base are assumed. These layers are determined by the ground failure pattern calculated from the individual soil layers. A berm adjacent to the foundation can be taken into consideration. The relation of the foundation thickness d to the foundation width b should not exceed 2 in this calculation.

Basis of calculation

Available standards

- DIN EN 1992-1-1/NA:2011/2012/2013/2015
- ÖNORM B 1992-1-1:2011
- NA to BS EN 1992-1-1/A2:2015/2009
- EN 1992-1-1:2004/A1:2014
- DIN EN 1997-1/A :2010
- ÖNORM B 1997-1:2013
- NA to BS EN 1997-1/A1:2014

National design standards

- DIN 1054 [2005-01]
- DIN 1054 [2010-12]
- DIN 4017 [2006-03]
- DIN 4019 [2014-01]
- ÖNORM B 4435-2 [1999-10] sowie
- J. Kanya / Bautechnik 05/1969

Basis of calculation in accordance with Kanya, Bautechnik 1969

Initial values

- a = foundation height
- b = foundation width
- c = load distance from outer edge of the foundation
- d = slab thickness
- l = clear distance between two neighbouring strip foundations
- E_b = modulus of elasticity of the concrete
- I_b = moment of inertia of a slab cross section with a width of 1 cm
- I_b = surface area of a slab cross section with a width of 1 cm
- E_{Bo} = stiffness modulus of the subsoil
- C_{Bo} = subgrade reaction modulus of the subsoil
- S_{Bo} = shear modulus of the subsoil
- P = resulting vertical load

Initial values – foundation restrained in the slab

$$\alpha = 3,2 \cdot \frac{E_b \cdot I_b}{l \cdot E_{Bo}}$$

$$\beta = \alpha - \frac{d}{2}$$

$$\gamma = \frac{P}{b}$$

$$\delta = \frac{2 \cdot \beta^2}{3 \cdot F_b \cdot E_b + 2,5 \cdot l \cdot E_{Bo}} \cdot F_b \cdot E_b$$

Special case – foundation restrained in the wall and the slab, wall pinned on top

$$\zeta = \left(\frac{I_{\text{Wall}}}{3 \cdot E_{\text{Wall}} \cdot I_{\text{Wall}}} \right) / \left(\frac{I_{\text{Slab}}}{4 \cdot E_{\text{Slab}} \cdot I_{\text{Slab}}} \right)$$

$$\alpha = 3,2 \cdot \frac{E_b \cdot I_b}{I \cdot E_{Bo}} \cdot \frac{1}{1 + \zeta}$$

$$M_{\text{Wall}} = \frac{\zeta \cdot M_Z}{1 + \zeta}$$

$$M_{\text{Slab}} = M_Z - M_{\text{Wall}}$$

Special case – foundation restrained in the wall and the slab, wall restrained on top

$$\zeta = \left(\frac{I_{\text{Wall}}}{4 \cdot E_{\text{Wall}} \cdot I_{\text{Wall}}} \right) / \left(\frac{I_{\text{Slab}}}{4 \cdot E_{\text{Slab}} \cdot I_{\text{Slab}}} \right)$$

$$\alpha = 3,2 \cdot \frac{E_b \cdot I_b}{I \cdot E_{Bo}} \cdot \frac{1}{1 + \zeta}$$

$$M_{\text{Wall}} = \frac{\zeta \cdot M_Z}{1 + \zeta}$$

$$M_{\text{Slab}} = M_Z - M_{\text{Wall}}$$

Exterior base pressure

$$\sigma_2 = \frac{\frac{2}{3} \cdot b^2 - c \cdot b + \delta + \alpha}{\frac{b^2}{6} + \delta + \alpha} \cdot \gamma$$

Interior base pressure

$$\sigma_1 = 2 \cdot \gamma - \sigma_2$$

Special case - gaping joint

$$b' = \frac{+c \pm \sqrt{c^2 + \frac{4}{3}(\delta + \alpha)}}{2} \cdot 3$$

$$\sigma_2 = 2 \cdot \gamma' = 2 \cdot \left(\frac{P}{b'} \right)$$

$$\sigma_1 = 0$$

Distance of the base pressure resultant from the outer edge of the foundation

$$s = \frac{1}{3} \left(\frac{\sigma_1}{\sigma_1 + \sigma_2} + 1 \right) \cdot b$$

Base pressure underneath the calculated equivalent area

$$\sigma' = \frac{(\sigma_1 + \sigma_2) \cdot b}{4 \cdot s}$$

Internal forces inside the centring plate

$$M_z = (\sigma_2 - \gamma) \cdot \alpha$$

$$H_z = (\sigma_2 - \gamma) \cdot \frac{\delta}{\beta}$$

Subgrade reaction modulus

$$C_{Bo} = 2,5 \cdot \frac{E_{Bo}}{b}$$

Angle rotation due to the centring moment

$$\phi = \frac{1}{4} \cdot \frac{M_z}{E_b \cdot I_b} \cdot l$$

Vertical displacement on the interior side

$$\Delta_1 = \frac{\sigma_1}{C_{Bo}}$$

Vertical displacement in the foundation centre

$$\Delta_v = \frac{\sigma_1 + \sigma_2}{2 \cdot C_{Bo}}$$

Vertical displacement on the exterior side

$$\Delta_2 = \frac{\sigma_2}{C_{Bo}}$$

Horizontal displacement on the bottom

$$\Delta_H = \phi \left(a - \frac{d}{2} \right) - \Delta_z$$

Horizontal displacement on the top

$$\Delta_z = \frac{H_z \cdot l}{F_B \cdot E_b}$$

Other parameters to be included

Self-weights of the foundation, the wall and the facing masonry

You can activate or deactivate separately the self-weights of the wall, the foundation and the facing masonry. Permanent loads always act simultaneously. The self-weight portions of the foundation, the wall and the facing masonry result from the defined values for the volume and the specific weight.

The self-weight is taken into account by generating a resulting load P , which is composed of the vertical loads of the respective load combination considering the associated combination rule and of the respective activated self-weight portions.

In this connection, a new resulting load distance C of the load P from the outer edge of the foundation is also calculated.

$$P = N_{Ed} + F_{Wall} + F_{facing} + F_{Foundation}$$

$$C = \frac{(N_{Ed} + F_{Wall}) \cdot l_{\text{distance to axis,Wall}} + F_{facing} \cdot l_{\text{distance to axis,Facing}} + F_{Foundation} \cdot l_{\text{distance to axis,Foundation}}}{N_{Ed} + F_{Wall} + F_{Facing} + F_{Foundation}}$$

Consideration of horizontal loads

In the calculation, horizontal loads are applied to the top of the foundation in the central axis of the wall. In the calculative approach of the software, they generate a moment with a lever arm that is as great as half the height of the connected reinforced concrete slab. The horizontal load itself is transferred through the foundation and considered in the design of the connection of the foundation to the reinforced concrete slab.

Consideration of moments

If moments are defined in addition to vertical loads or if moments result from the horizontal loads at the base of the wall, they influence the position of the resultant of the vertical loads. Moments defined as positive rotate the foundation clockwise towards the inside of the building. The resultant of the vertical loads is displaced by the length $e = M_{Ed} / P$ towards the inside of the building. Moments defined as negative act inversely because of the negative sign of e .

Data entry

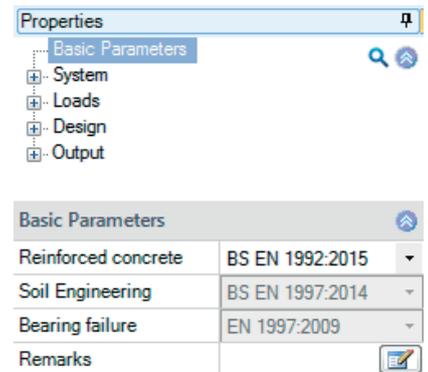
You can enter values and define control parameters in the menu on the left screen section. The effect of the entered values is immediately shown in the graphical representation on the right screen section. Before entering any data, you can edit the dimensional units (cm, m ...) via the options File ▶ [Program settings](#).

Wizard

The [definition wizard](#) is automatically launched when you start the software. You can disable the wizard in the settings menu.

Input options in the three-dimensional GUI

The data entry via the GUI is described in the document "[Basic operating instructions-PLUS](#)."



Basic parameters

Reinforced Concrete

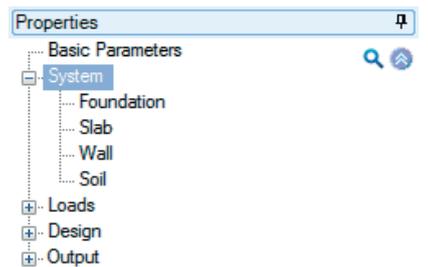
Select the desired reinforced concrete standard:
See [Basis of calculation](#).

Foundation engineering and ground failure

According to the selected reinforced concrete standard, the software selects the corresponding standards for foundation engineering and ground failure automatically.

Comments

Click on the  button to enter your own comments on the system.

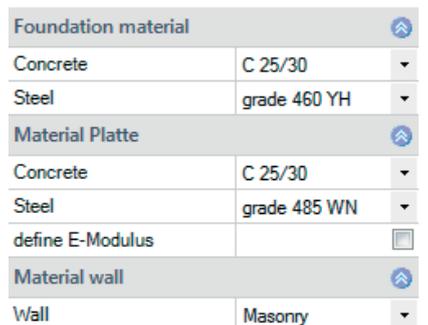


Structural system

Material

Select the quality of the concrete and the reinforcing steel grade as well as the material of the wall (masonry or concrete).

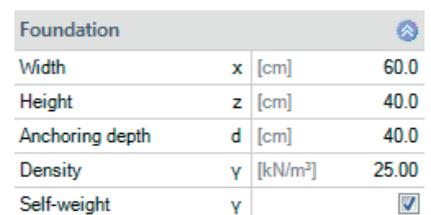
For the calculation of internal forces, of soil pressure and base pressure as well as of deformations, the modulus of elasticity of the connected reinforced concrete slab is used as a standard. Optionally, you can specify a user-defined modulus.



Foundation

In the foundation ground plan, the x-axis (positive) runs from the left to the right and the y-axis (positive) from the bottom to the top.

Width	x	foundation dimension in x-direction
Height	z	foundation height
Anchoring depth d		lowest foundation depth below the ground level or below the top edge of the basement floor.
Specific weight γ		gamma concrete
Self-weight		automatic inclusion of the self-weight of the foundation.



Foundation			
Width	x	[cm]	60.0
Height	z	[cm]	40.0
Anchoring depth	d	[cm]	40.0
Density	γ	[kN/m ³]	25.00
Self-weight			<input checked="" type="checkbox"/>

Slab

- Connection** the connection of the slab to the foundation can be flexurally rigid or pinned.
- Factor EI** when selecting "flexurally rigid", it is the factor for the bending stiffness of the slab. It refers to the stiffness of the cross section in state I.
- Top load...** when selecting "pinned joint", it refers to the permanent load portion of the reinforced concrete slab that is supposed to act on the foundation.
- Width** clear distance between the foundations.
- Height** height of the slab.

Slab		
Connection		flexural rigid
factor EI		flexural rigid
Width	y [cm]	400.0
Height	z [cm]	15.0

Wall

- Fixing on top** specifies how the wall impedes torsion of the foundation:
pinned, restrained, no fixing.
- Factor EI** pinned joint: factor for the flexural rigidity of the slab. It refers to the stiffness of the cross section in state I.
- Thickness x** thickness of the wall.
- Height z** height of the wall.
- Specific weight y** specific weight of the wall.
- Self-weight** automatic inclusion of the self-weight of the foundation.
- Eccentricity, transverse** eccentricity in the x-direction

Wall		
Fixing head		hinged
factor EI		hinged
Thickness	x	restrained
Height	z [cm]	300.0
Density	γ [kN/m ³]	25.00
Self-weight	γ	<input type="checkbox"/>
Eccentricity	across [cm]	-15.0
Clinker		
Clinker		without clinker
Thickness	x [cm]	10.0
Height	z [cm]	300.0
Density	γ [kN/m ³]	25.00
Self-weight	γ	<input type="checkbox"/>
Eccentricity	across [cm]	0.0

Clinker / Wall facing

- Clinker** allows you to select whether facing masonry should be included in the calculation.
- Thickness** thickness of the facing masonry.
- Height** height of the facing masonry.
- Specific weight** specific weight of the soil.
- Self-weight** activate this option to include the self-weight of the facing masonry automatically.
- Eccentricity** eccentricity of the facing masonry in the transverse direction.

Soil

Soil properties

- Base pressure select whether to define the design value of the base pressure resistance by entering a user-defined value, consulting a standard table or determining it in a self-defined table - see the paragraph below.
- Base pressure resistce. permissible base pressure $\sigma_{R,d}$
- Friction angle φ' friction angle of the drained soil underneath the foundation base.
- Perm. V permissible displacement. It is compared to the maximum displacement of the foundation in the vertical direction.
- Stiffness modulus $E_{s,min}/max.$; upper/lower limit of the stiffness modulus. You can define the upper and lower limits for the stiffness modulus. In each superposition, the most unfavourable values are used. If the calculation should be performed without limits, enter the same value for the upper and the lower limit. The stiffness modulus is provided by the soil expert. Betonkalender 1998, part 2, p. 472 specifies guiding values for the stiffness modulus E_s in MN/m^2 : gravel, pure: 100.0 to 200.0 - sand, pure: 10.0 to 100.0 - coarse clay: 3.0 to 15, clay 1.0 to 60.0 - peat 0.1 to 1.0
- Groundwater check this option when groundwater exists in the subsoil - the data-entry field for the depth of the ground water is displayed subsequently.
- Groundwater depth absolute depth of the groundwater below the bottom edge of the foundation body.
- Soil bearing resistance from table if you do not enter a user-defined value for the soil pressure, the design value is obtained either from a standard table or a user-defined table.

Soil properties		
Bearing pressure		DIN 1054:2015
Bearing resistance	$\sigma_{R,d}$ [kN/m ²]	350.00
Friction angle	φ' [°]	30.00
admissible deformations adm. V	[cm]	1.0
Stiffness value	$E_{s,min}$ [MN/m ²]	11.50
Stiffness value	$E_{s,max}$ [MN/m ²]	11.50
Groundwater Exists		<input checked="" type="checkbox"/>
Groundwater Depth	[m]	0.00
Soil bearing resistance by table		
First soil layer		
Density	γ [kN/m ³]	18.5
Density by buoyancy	γ' [kN/m ³]	11.0
Friction angle	φ' [°]	30.0
Cohesion	c' [kN/m ²]	0.0
Soil layers		

The button accesses the table definition dialog.

Bearing resistance		
According to Annex		Table A6
Konsistenz		rigid
Increase (geometry)	20 %	<input type="checkbox"/>
Increase (strength)	50 %	<input type="checkbox"/>
Anchoring depth	d [cm]	40.0
Groundwater Exists		<input type="checkbox"/>

Definition parameters for the standard table:

- From annex of standard selection of the table in the selected soil standard or the currently active NAD. The permissible base pressures are taken from this table.
- Consistency consistency of the soil: rigid, semi-solid, solid.
- Increase (geometry) the permissible soil pressure can be increased by 20 % if the corresponding border conditions (b/d) specified in the standard are complied with.
- Increase (strength) optional increase by 50 % if the soil is sufficiently solid.
Note: The values are added up under particular conditions (70 %).
- Anchoring depth d lowest foundation depth below the ground level or the top edge of the basement floor.

Define table:

To enter the design value of the base pressure resistance, you can add a new row to the self-defined table by pressing the  button. This value should be taken from a soil expertise and should ensure sufficient safety against ground failure and a sufficient limitation of the settlement. Enter the corresponding foundation width and anchoring depth in addition. The functions of the buttons are explained in the tool tips.

First soil layer

The first soil layer is entered directly. Additional soil layers can be added to the table via the “soil layers” button .

First soil layer			
Density	γ	[kN/m ³]	18.5
Density by buoyancy	γ'	[kN/m ³]	11.0
Friction angle	φ'	[°]	30.0
Cohesion	c'	[kN/m ²]	0.0
Soil layers			

Specific weight γ specific weight of the soil.

Spec. weight under buoyancy γ' specific weight of the soil layer under buoyancy. Define [ground water](#) to enable this data-entry field.

Friction angle φ' friction angle of the soil in this layer.

Cohesion c' soil cohesion.

Additional soil layers / additional parameters

	γ	γ'	φ'	c'	xU	other
	[kN/m ³]	[kN/m ³]	[°]	[kN/m ²]	[cm]	
→ 1	18.5	11.0	30.0	0.0	150.0	

xU thickness of the soil layer. You cannot define soil layers with less than 0.1 m thickness.

Activating the  button accesses a dialog with additional parameters:

Calculation of the settlement

Method user-defined / based on stiffness modulus
Select whether to enter a user-defined value for the modulus of compressibility E^* or have it calculated from the stiffness modulus and the correction factor (DIN 4019 P1).

E^* modulus of compressibility. The compressibility of the soil can be obtained from a pressure settlement line or be calculated using the stiffness modulus and the correction factor.

E_s stiffness modulus

x correction factor

Soil layer			
Settlement analysis			
Procedure			direct specification
Compresses modulus	E^*	[kN/m ²]	4946.0
Stiffness module	E_s	[kN/m ²]	9892.0
Correction factor	x		0.50
Settlement analysis: Consolidation			
Permeability factor	k	[m/s]	1E-09
Both sides drained			<input type="checkbox"/>
Darstellung			
Color			automatically

Calculation of settlement, consolidation

k coefficient of permeability for the consolidation speed. You can take the value from a soil expertise.

Both sides drained for the calculation of the time until the consolidation settlement subsides approximatively, the full layer thickness is taken into account with drainage on one side. With drainage on both sides, only half of the layer thickness is taken into account.

Representation

Colour you can define the colour of the soil layer according to your wishes.

Soil type options ranging from "coarse gravel" to "rock"

Soil aggregates 1/2 options ranging from "coarse granular" to "blocky"

Note: The selection of the soil type is only important for the colour representation on the graphical screen.

Loads

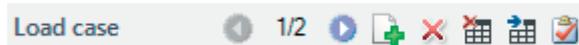
Delete horizontal loads: The  button allows you to delete all horizontal loads at once! This function can be helpful when importing load cases from other software applications. You can define horizontal loads in the load / load cases dialogs.

Load cases

Enter the data of the first load case either in the corresponding data-entry mask or directly in the load case table, which you can display

below the graphic by activating the  button.

Load case toolbar:



To add load cases, always set up a new load case first by activating the  button (a data-entry mask for the new load case is displayed each time).

- See also [Data entry via tables](#) in the Basic operating instructions PLUS.pdf

Tip: A description is displayed in the status line each time you click into a particular data-entry field.

Load Cases	
Load case	1/2
Wall loads characteristic	
Description	Load case 1
Action	Permanent loads
Vertical force in z	[kN] 60.0
Moment about y across	[kNm] 0.00
Horizontal Force in x across	[kN] 0.0
Group affiliation	
Simultaneous group	0
Alternative group	0

Characteristic wall loads

Description optional text to the selected action can be entered. This text is included in the output.

Action the appropriate actions can be selected from a list:
Permanent loads ... Earthquake.

Axial force in z vertical force N_z in the centre of the wall

Moment about y moment $M_{y,trans}$ defined with for the wall.

Horizontal force in x horizontal loads act on the top edge of the foundation. These horizontal loads generate moments on their way down to the foundation base, which are taken into account automatically.

Description	Act	N_z [kN]	$M_{y,quer}$ [kNm]	$H_{x,quer}$ [kN]	SIM	ALT
1 Load case 1	Permanent loads	60.0	0.00	0.0	0	0
2 Load case 2	Permanent loads	60.0	0.00	0.0	0	0

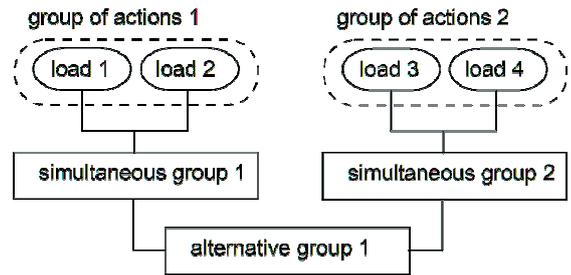
Cat. A: domestic, residential areas
 Cat. B: office areas
 Cat. C: congregation areas
 Cat. D: shopping areas
 Cat. E: storage areas
 Cat. F: traffic $F \leq 30$ kN
 Cat. G: traffic 30 kN $< F \leq 160$ kN
 Cat. H: roofs
 Wind loads

Grouping

Concurrent group

Loads of a particular action group can be defined as "always acting simultaneously"

Ill.: *Example for the functioning of alternative and concurrent groups*



Alternative group

Different variable load cases with similar actions can be combined to an alternative load case group by assigning an alternative group number to them. Only the decisive load case of this alternative load case group is invoked in the superposition.

Base pressure

Displaying the base pressure pattern

To ensure traceability, the base pressure pattern with stress ordinates (red and blue arrow icons in the multifunction bar) can be displayed in the 3D representation of the load arrangement for all load cases and superpositions decisive in the verifications. Click on the base pressure icon to activate the function. The graphic is displayed in a pop-up window.



Design

Settings

Minimum reinforcement	ductility reinforcement in accordance with the selected reinforced concrete standard
Earthquake: $\Psi_2=0.5$	in accordance with the introductory decree of DIN 4149 for Baden-Württemberg, the combination coefficient $\Psi_2 = 0.5$ for snow loads should be used in the superpositions with earthquake loads.
Shear force as beam	specification whether the shear resistance should be verified on a slab or a beam.
Min. eccentricity	Considering minimum eccentricities for compression member by EN 1992-1-1 6.1 (4).
Minimum reinforcement for column	this option allows you to take a minimum reinforcement for compression members into account.
Fix mats transversely	Ticked option means: the mats in the transverse direction are also taken into account for the increase of the predefined reinforcement.

Settings	
Minimum of reinforcement	<input checked="" type="checkbox"/>
Earthquake: $\Psi_2=0.5$.	<input type="checkbox"/>
Shear force as beam	<input type="checkbox"/>
Min. eccentricity	<input checked="" type="checkbox"/>
Minimum reinforcement for column	<input checked="" type="checkbox"/>
Fix mats transversely	<input checked="" type="checkbox"/>

Reinforcement

The software allows you to define non-specifically up to two layers of fabric and two layers of bar steel distributed over the entire foundation on top and bottom.

Cv	concrete cover at base
Cnom,S	concrete cover in general
Reinforcement layer:	
Found. bottom	reinforcement layer on the bottom of the foundation.
Slab top/bottom	reinforcement layer on the top/bottom of the slab.
Diam. longitud. reinf.	selection list of the diameter that shall be used for the generation of the longitudinal reinforcement. The software generates sufficient reinforcement of this diameter to comply with the required reinforcement. If the minimum and maximum spacing cannot be realised with the defined diameter, a greater diameter is used.
Durability	activating the  button displays the corresponding Durability dialog. When you confirm your settings in this dialog with OK, the concrete cover, reinforcement layers and their diameter are checked and adjusted accordingly.
Creep and shrinkage	displays the dialog to define the creep factor and the shrinkage strain .
Distribution	displays the enhanced reinforcement dialog for top/bottom/slab.
Delete reinforcement	deletes the default reinforcement.
Practical construction spacing	as a standard, the bar spacing is calculated accurately, i.e. the resulting bar spacing is calculated precisely to the millimetre. If the option is ticked the bar spacing is adjusted to 5, 6, 7, 7.5, 8, 9, 10, 12.5, 15, 17.5, 20, 22.5, 25, 27.5 or 30 cm.

Properties	
Basic Parameters	
System	
Loads	
Design	
Reinforcement	
Soil Engineering	
Output	

Reinforcement			
Concrete cover base	cv	[cm]	3.0
Concrete cover general	cnom,S	[cm]	3.0
Layer of reinforcement	Bottom base	[cm]	3.7
Layer of reinforcement	Platte oben	[cm]	3.0
Layer of reinforcement	Plate bottom	[cm]	3.0
Longitudinal diameter		14 mm	
Durability	Foundation	XC2/X0	
Durability	Slab	XC2/X0	
Creep and shrinkage	Slab	XC2/X0	
Distribution			
Delete reinforcement			
Practical construction spacing			<input checked="" type="checkbox"/>

Enhanced reinforcement dialog



The enhanced reinforcement dialog can be accessed via the  button or via **Design** ▶ **Reinforcement** ▶ **Distribution**.

Next to the tabs for the lower and the upper reinforcement the “Slab” tab is displayed.

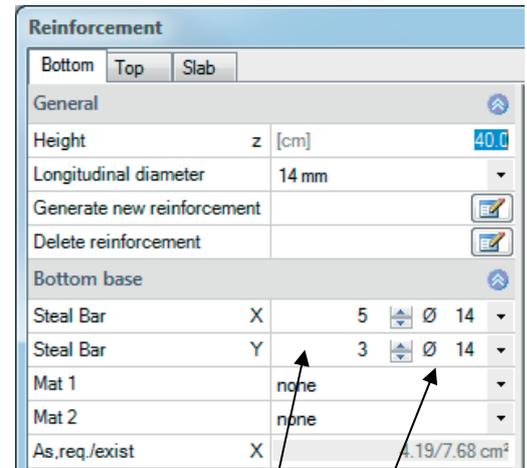
General

Height height of the foundation in the x-direction.

Diam. longitud. reinf. as described for the [Reinforcement](#) dialog.

Generate new reinforcement the software calculates a reinforcement that satisfies the requirements as a minimum. If the minimum and maximum spacing cannot be realised with the selected diameter, a greater diameter is used. When you delete the default reinforcement, the automatic generation of the reinforcement is disabled and the default reinforcement remains unchanged. If it turns out to be insufficient, the software displays a warning message. If no reinforcement was pre-set, no warning is displayed. When generating the reinforcement automatically, the software starts with the default longitudinal diameter.

Delete reinforcement deletes the default reinforcement and the structurally required reinforcement is used in the calculations.



Reinforcement			
Bottom Top Slab			
General			
Height	z [cm]		40.0
Longitudinal diameter			14 mm
Generate new reinforcement			
Delete reinforcement			
Bottom base			
Steel Bar	X	5	∅ 14
Steel Bar	Y	3	∅ 14
Mat 1		none	
Mat 2		none	
As, req./exist	X		4.19/7.68 cm²

Number / Diameter

Foundation bottom/top/slab

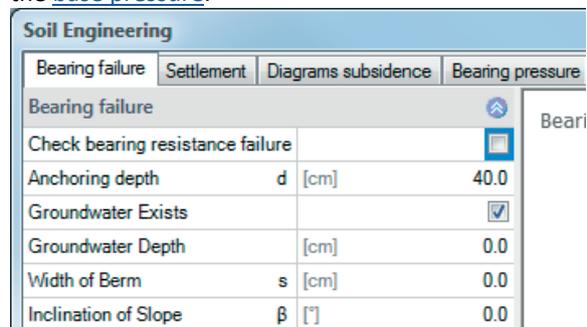
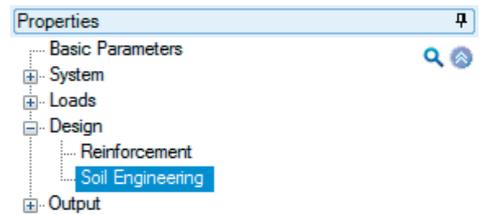
Bar steel X/Y define the number of bars in the first column and the diameter of the bars in the second column separately for the x-direction and the y-direction.

Fabric 1/2 selection of a reinforcement steel fabric

As, req./exist. informative indication

Soil Engineering

Simplified verification	the verification in the limit states of the ground failure, the slide stability and the serviceability (verification of the settlement) are replaced by empirical design values for the base pressure resistance.
Perform slide stability analysis	if the load vector is not perpendicular to the base surface the foundations must be examined for failure due to sliding in the base surface.
Perform ground failure analysis	in the ground failure analysis, the shear resistance of the soil underneath the foundation plane is taken into account. The soil layers above the foundation plane are included in the form of top loads if the base surface and the ground top surface are horizontal.
Calculate settlement	in the calculation of the settlement, the compressibility of the soil down to the settlement influence depth t_s is to be considered. This depth can be assumed at the level at which the perpendicular additional stress of the average effective settlement load equals 20 % of the effective perpendicular initial stress of the soil.
Soil engineering analyses	the option allows you to access the enhanced soil engineering dialog with graphical representations of the ground failure, settlements and the base pressure .



Ground failure

Anchoring depth d	lowest foundation depth below the ground level or below the top edge of the basement floor.
Groundwater	this option allows you to define whether groundwater exists in the area or not.
Groundwater depth	absolute depth of the groundwater below the bottom edge of the foundation body.
Berm width s	the berm width is the distance from the outer edge of the foundation to where the slope starts.
Slope inclination β	the inclination angle of the ground surface starting at the defined berm. The inclination angle has an influence on the ground failure analysis. This angle only refers to downward gradients.

Settlements

Calculate settlements in the calculation of the settlement, the compressibility of the soil down to the settlement influence depth t_s is to be considered. This depth can be assumed at the level at which the perpendicular additional stress of the average effective settlement load equals 20 % of the effective perpendicular initial stress of the soil.

Settlement with permanent loads only select whether the settlements should be calculated only with permanent loads or with permanent and variable loads.

Output

Scope of the output and options

By checking the desired options, you can determine the scope of texts to be put out. You can adjust the font size and the scale of the graphics to be put out.

Output as a PDF file

On the “Document” tab, a PDF document is displayed.

See also the document [Output and printing](#).

Properties

- Basic Parameters
- System
- Loads
- Design
- Output
 - General
 - Soil Engineering
 - Reinforced concrete

Output

Output scope	User defined
General	
Font height	Output 0.25 cm
Scale	1:33
Scale details	<input type="checkbox"/>
Coordinate system in the output	<input type="checkbox"/>
Load Cases	<input type="checkbox"/>
Superposition	<input type="checkbox"/>

The screenshot shows a software window with a 'Document' tab. The main content is a PDF document titled '#Company#' and 'Item: (Untitled)'. The document contains the following sections:

Reinforced Raft Foundation FDR+ 02/2016 (Friolo prerelease, 5/17/2016)

System Graphics

Reinforced raft foundation acc.to Kenya [Bautechnik 5/1969] in conjunction with DIN EN 1992-1-1/NA Berichtigung 1:2012-06 and DIN EN 1997-1/NA:2010-12

System Values

Member	Concrete	Steel	Width (x) cm	Height (p) cm
wall (Masonry)	C 25/30	B500A	30.0	300.0
Foundation	C 25/30	B500A	60.0	40.0
Plate	C 25/30	B500A	400.0	15.0

Eccentricity relative to the wall axis wall ex = -15.0cm, using rigidities in state Ix reduction Factor! Plate restrain: 1.00 x 4 x EI_{wall}/4.00. To accommodate the friction is to avoid the use of insulation and the like under the base plane. Stiffness value E_s = 11.50 MN/m². Anchoring depth d = 40.0cm. Grundwasser oberhalb der Sohle 0.0cm.

Actions (Act)

Act	Name	ψ ₀	ψ ₁	ψ ₂	simultaneous load cases
A	cat. A: domestic, residential areas	0.70	0.50	0.30	2
g	Permanent loads	1.00	1.00	1.00	1

characteristic load cases

No.	Act	Description	N kN	M _y kNm	H _x kN	SIM	ALT
1	g	Load case 1	60.0	0.0	0.0	0	0
2	A	Load case 2	60.0	0.0	0.0	0	0

Horizontal loads according at upper edge of foundation. Foundation 6.00 kN (considered), wall 16.20 kN (unconsidered).

Superpositions

No.	DS	Superposition
1	P	1.35 x (1) + 1.5 x (2)
2	P	1.0 x (1) + 1.0 x (2)
3	P	1.0 x (1)
4	P	Self-weight
5	P	1.0 x (1) + 0.3 x (2)

BS: design situation P: Permanent
The load case numbers are listed in parentheses.

Result Overview Verifications