

# ST13 Shear Panel Stiffness

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**Shear field**

Trapez. plate pr.  >>  
(positive position)

Beam cr. section  >>

Loading

Number fields n= 4 Stk.

Span length l0= 6.00 m

Height of field Hs= 12.00 m

Shear field from  to

**Foundation constants**

Shear field found. S= 17246 kN

Transl. foundation cy= 14560 kN/m<sup>2</sup>

Rotary found. c theta= 4.49 kNm/m

**verification of sufficient obstruction...**

of lat. displacmt. S min= 10636 < S vorh

of rotation (E-E) c theta min= 0.00 < c theta exist

of rotation (E-P) c theta min= 0.00 < c theta exist

Text output

Select a steel trapezoid plate profil. (F5 or click on button '>>')

23.06.2017 | 10:32



# Shear Panel Stiffness ST13

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Further information and descriptions are available in the relevant documentations:

<a href="#">FDC – Basic Operating Instructions</a>	General instructions for the manipulation of the user interface
<a href="#">FDC – Menu items</a>	General description of the typical menu items of Frilo software applications
<a href="#">FDC – Output and printing</a>	Output and printing
<a href="#">FDC - Import and export</a>	Interfaces to other applications (ASCII, RTF, DXF ...)
<a href="#">FCC</a>	Frilo.Control.Center - the easy-to-use administration module for projects and items
<a href="#">FDD</a>	Frilo.Document.Designer - document management based on PDF
<a href="#">Frilo.System.Next</a>	Installation, configuration, network, database

## Application options

For beams under bending stress, there is always a risk of lateral shift and torsion. The examination of lateral torsional buckling is based on the assumption that the lateral shift  $v$  and the torsion  $\vartheta$  can occur independently of each other.

In many cases, structural parts such as trapezoidal steel sheeting are connected to the beams that provide elastic support. You can map the effect of stabilising components in the calculation through rotational springs  $c_\vartheta$  and through the ideal shear stiffness  $S_i$ . The total prevention of the lateral shift  $v$  at the distance  $f$  from the shear centre  $M$  constitutes a limit case, in which we speak of as “fixed axis of rotation”.

The stabilising effect of the trapezoidal steel sheeting on bending beams can be considered from two different points of view. On the one hand, the fixity against lateral shift and torsion can be proven by establishing evidence of sufficient shear stiffness and torsional restraint. In this case, a verification of the beam's resistance to lateral torsional buckling is not required. On the other hand, it is permitted to consider the effective shear stiffness and the effective torsional restraint in the determination of the ideal lateral torsional buckling moment  $M_{ki,y}$  instead of performing the afore-mentioned verification. The verification of the resistance to lateral buckling must be performed accordingly in this case.

The **ST13** application calculates the rotational spring  $c_\vartheta$  [kNm/m], the ideal shear stiffness  $S$  [kN] as well as the translational restraint  $c_y$  [kN/m<sup>2</sup>]. These values allow you to take the stabilising effect of trapezoidal steel sheeting into account. In addition to this, the application verifies the fixity against lateral shift and torsion. If the verification is not successful, an additional lateral stability verification is required. In practice, the verification whether the torsional restraint is sufficient is hardly ever successful. A lateral torsional buckling analysis is required in most cases. The spring constants calculated by ST13 can be transferred to the relevant applications such as BTII.

## Basis of calculation

See the document [ST13 Basis of Calculation.pdf](#) (Only in German).

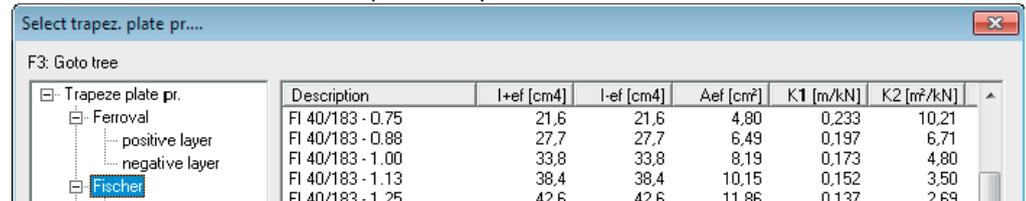
## Data entry

### Definition of the structural system

#### Shear panel

##### Trapezoidal steel sheeting

Click on the button  to access the dialog 'Select trapezoidal sheeting...'. The sheeting types of various manufacturers are listed in this section. The calculation of the restraint constants of a selected trapezoidal steel sheeting is based on the border conditions specified by the user.



Description	I-ef [cm4]	I-ef [cm4]	Aef [cm²]	K1 [m/kN]	K2 [m²/kN]
FI 40/183 - 0.75	21,6	21,6	4,80	0,233	10,21
FI 40/183 - 0.88	27,7	27,7	6,49	0,197	6,71
FI 40/183 - 1.00	33,8	33,8	8,19	0,173	4,80
FI 40/183 - 1.13	38,4	38,4	10,15	0,152	3,50
FI 40/183 - 1.25	42,6	42,6	11,86	0,137	2,69

After selecting a trapezoidal steel sheeting, the position (positive, negative) in which it is to be used is shown below the display field.



The selection of the trapezoidal steel sheeting determines implicitly whether the profile is used in the positive or negative position.

*Note: You can access this selection dialog also via the menu item 'Trapezoidal sheeting'.*

#### Beam profile

Click on the button  to access the dialog [Select - edit cross section](#). A list of the available cross-sections is displayed. When selecting a cross-section, the calculation of the restraint constants is based on the border conditions specified by the user.

*You can access this selection dialog also via the menu item 'Profile - beam'.*

#### Loads

type of loading. The following options are available for selection:

- structural load; the trapezoidal steel sheeting is pressed against the beam under bending stress
- suction; the trapezoidal steel sheeting lifts up from the beam under bending stress

#### Number of spans

the total span is defined via the number of partial spans  $n$  with the span length  $l_0$ . Consequently, the resulting total length is  $l_0 \cdot n$ .

#### Span length

length  $l_0$  of a partial span in [m];  
corresponds to the spacing of the components to be strutted (e.g. spacing of portal frames).

#### Span height

height  $H_s$  of the shear panel in [m];  
corresponds to the length of the component to be strutted (e.g. length of the vertical member of a portal frame)

#### Shear panel

**from** axis (0 to  $n$ ), at which the structurally effective shear panel starts.

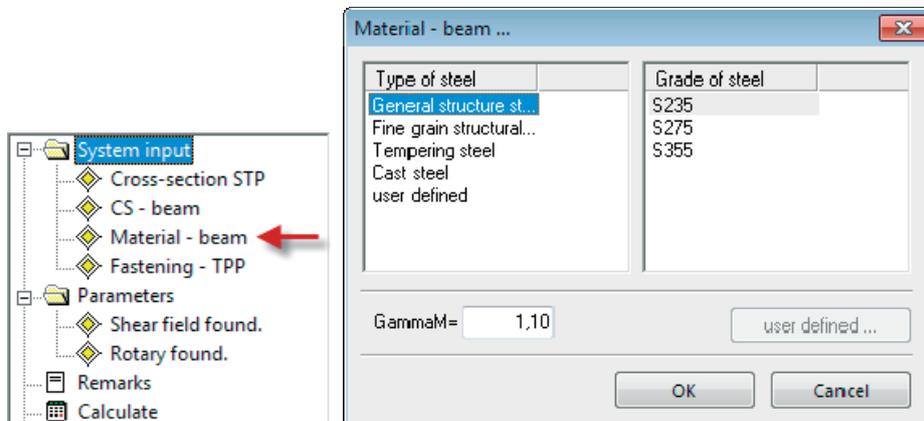
**to** axis (0 to  $n$ ), at which the structurally effective shear panel ends.

These specifications are used to determine the number of components to be strutted by the shear panel.

The numbering of the axes starts with zero and is indicated in the graphical representation.

## Material - beam

Access the material definition dialog by double-clicking on the item 'Material - beam' in the main menu. You can select the material as per DIN 18800 part 1, table 1, from the displayed lists (type, grade) or enter user-defined values. The software assumes a constant module of elasticity  $E_k$  and a constant yield stress  $f_{yk}$  over the total beam.



### Material

First select the type of steel and then the grade.

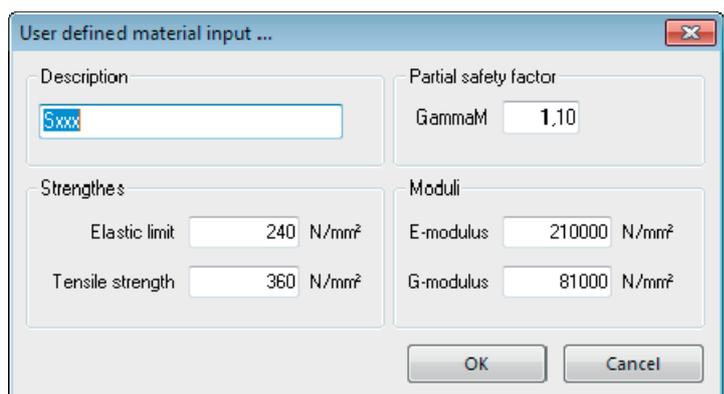
For the available steel types, the characteristic values of the moduli of elasticity and shear are considered in the calculation. In connection with the stress verification, you should note, that the characteristic value of the yield strength  $f_{yk}$  is set to the standard value, the product thickness of the cross-sections must be taken into account, however, and must be reduced if required.

### User-defined material parameters

Click on 'User-defined' in the type menu and then on the 'User-defined' button.

Enter a name for the material and the characteristic values for the yield strength  $f_{yk}$  and the tensile strength  $f_{tk}$  as well as for the modulus of elasticity  $E_k$  and the shear modulus  $G_k$  an.

If you specify material parameters that correspond to a standard steel according to DIN EN, the values are reset to those of the corresponding type and grade.



## Fasteners - trapezoidal steel sheeting

### Bolts in...

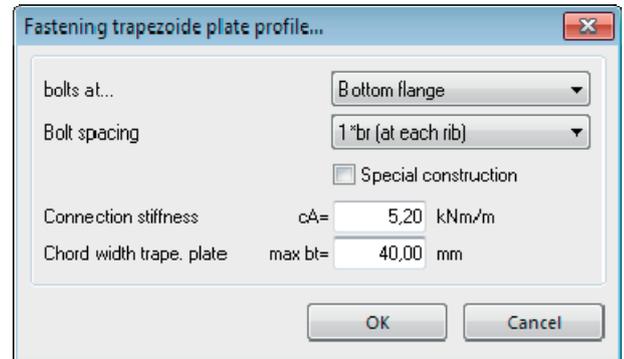
Select the type of fasteners from the selection list:

- bottom flange, the trapezoidal steel sheeting is fixed to the lower flange
- top flange, the trapezoidal steel sheeting is fixed to the upper flange

### Bolt spacing

Select the desired bolt spacing from the selection list:

- $1 \cdot b_r$ ; the trapezoidal steel sheeting is fixed at each profile rib.
- $2 \cdot b_r$ ; the trapezoidal steel sheeting is fixed at every second profile rib.



### Special versions

Check this options when the trapezoidal steel sheeting is fixed in accordance with DIN 18807-3, figure 7.

**c<sub>A</sub>** connection stiffness resulting from the border conditions specified by the user. The connection stiffness is part of the [torsional restraint](#).  
→ See [Basis of Calculation](#).

**max b<sub>t</sub>** maximum permissible flange width of the trapezoidal steel sheeting, which results from the border conditions specified by the user.

## Shear-panel restraint - parameters

**K1/K2** shear panel values in accordance with the building inspection approval for the configuration of the fasteners in accordance with DIN 18807-3, figure 7 for the calculation of the ideal shear modulus in [m/kN] / [m<sup>2</sup>/kN].

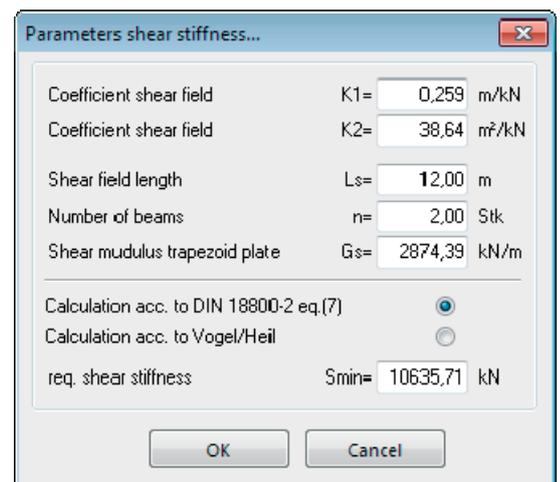
**L<sub>s</sub>** length of the structurally effective shear panel in [m].

**n** number of components to be strutted.  
Please keep in mind that the edge beams are only included half in the calculation.

**G<sub>s</sub>** ideal shear modulus in [kN/m].

### Calculation as per DIN 18800-27, eq. (7)

Check this option if the minimum shear stiffness is to be calculated with equation (7) DIN 18800-2.



### Calculation in accordance with Vogel/Heil

Check this option if the minimum shear stiffness is to be calculated in accordance with [Vogel/Heil](#) [9], page 232.

**S<sub>min</sub>** required shear stiffness, an additional examination of lateral torsional buckling of the beam under bending stress is not required (fixed axis of rotation)

## Parameters for the calculation of the torsional restraint

### Moment coefficient for the verification of a sufficient torsional restraint ...

- Free axis of rotation** coefficient  $k_{theta}$  as per DIN 18800-2, table 6, column 2.  
A free axis of rotation must be assumed if the condition formulated in equation (7) of DIN 18800-2 is not satisfied. See [Basis of Calculation](#).
- Fixed axis of rotation** coefficient  $k_{theta}$  as per DIN 18800-2, table 6, column 3.  
A fixed axis of rotation may be assumed if the condition formulated in equation (7) of DIN 18800-2 is satisfied. See [Basis of Calculation](#).

### Torsional restraint through bending stiffness of the component to be strutted

- k** system coefficient  
 $k = 2$  for single-span and double-span beams  
 $k = 4$  for three-span and multi-span beams
- $E_k$**  characteristic value in [kN/cm<sup>2</sup>] of the modulus of elasticity .
- $I_a$**  area moment of inertia of the trapezoidal profile in [cm<sup>4</sup>].
- a** spacing of the components to be strutted (beams) in [m].
- c theta M** characteristic value in [kNm/m] for the torsional restraint provided by the bending stiffness of the trapezoidal steel sheeting.

### Torsional restraint provided by the deformation of the connection

- c'theta A** characteristic value in [kNm/m] for the connection stiffness  $\bar{C}_{\theta A,k}$  of trapezoidal steel sheeting referenced to a flange width of 100 mm as per DIN 18800-2, table 7.

Click on the  button to access the dialog 'Torsional restraint as per DIN 18800-2, table.7...'

The displayed dialog corresponds to table 7 of DIN 18800-2.

The concrete value for the characteristic connection stiffness of trapezoidal steel sheeting

depends on the border conditions specified by the user and is referenced to a flange with of 100 mm.

You should note in this connection that when selecting a different value in column 'c', also different border conditions are required as a basis. When you confirm your selection with 'OK', the corresponding new border conditions are applied automatically to the structural system.

Line	Trapeze pr. layer		Bolts in		bolt spacing		Washer diameter	c	max bt
	Positive	Negative	Bottom fl	Top flang	br	2*br			
<b>Loading</b>									
1	x		x		x		22	5,20	40
2	x		x			x	22	3,10	40
3		x		x	x		Ka	10,00	40

**b** flange width of the beam profile in [cm].

#### Always include ctheta A

Check this option if the resilience of the connections should always be considered (as required by Lindner [6]).

Uncheck this option if the software should check whether the resilience of the connections can be disregarded because of the contact moment (in accordance with Krüger [7]).

#### Load-bearing capacity qtz

load-bearing capacity of the beam under bending stress in [kN/m].

**ctheta A** characteristic value in [kNm/m] for the torsional restraint resulting from the deformation of the connection.

#### Rotational restraint resulting from the deformation of the profile

**h** profile height in [cm] of the beam under bending stress.

**s** web thickness in [cm] of the beam under bending stress.

**b** flange width in [cm] of the beam under bending stress.

**t** flange thickness in [cm] of the beam under bending stress.

**c<sub>1</sub>** for I-shapes under structural load or suction load  $c_1 = 0.5$

For C-shapes under structural load  $c_1 = 0.5$

For C-shapes under suction load  $c_1 = 2.0$

**ctheta P** characteristic value in [kNm/m] for the torsional restraint resulting from the deformation of the beam profile.

## Calculation and results

### Restraint constants

Shear panel restraint S	ideal shear stiffness in [kN]
Translational restraint $c_y$	translational restraint in [kN/m]
Torsional restraint c	torsional restraint in [kNm/m]

### Verification of sufficient shear stiffness and torsional restraint

Of lateral shift $S_{min}$	verification of the minimum shear stiffness in [kN] as per DIN 18800-2, eq. (7) or in accordance with reference [7].
Of the torsion (e - e) c	verification of the minimum torsional restraint as per DIN 18800-2, eq. (8) for the verification method e - e; in [kNm/m].
Of the torsion (e - p) c	verification of the minimum torsional restraint as per DIN 18800-2, eq. (8) for the verification method e - p; in [kNm/m].

Foundation constants	
Shear field found.	S= 17246 kN
Transl foundation	$c_y$ = 14560 kN/m <sup>2</sup>
Rotary found.	c theta= 4,49 kNm/m
verification of sufficient obstruction...	
of lat. displacem.	S min= 10636 < S vorh
of rotation (E-E)	c theta min= 0,00 < c theta exist
of rotation (E-P)	c theta min= 0,00 < c theta exist
Text output	

## Output

Output of the system data, results and graphical representations on the screen or the printer.

Word                    output to MS-Word, if this software is installed on the computer.

Screen                 displays the data in a text window on the screen

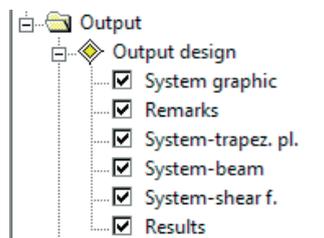
[Print](#)                   starts the output on the printer

### Comments

The 'Comments' item in the main menu allows you to enter user-defined texts that are included in the output.

## Output profile

The dialog offers comprehensive options for the control of the output scope. Check the items to be put out.



File >> Page view displays a print preview as a PDF

## Reference literature

- [1] DIN 18800-2
- [2] Stahlbauten-Erläuterungen zu DIN 18800 Teile 1 bis Teil 4
- [3] DIN 18807, Part 1 to Part 3
- [4] Stahltrapezprofile, 2. Auflage, Maaß, Hünersen und Fritzsche, Werner Verlag 2000
- [5] Stabilisierung von Biegeträgern durch Trapezbleche, Stahlbau 56 (1987), p. 9-15
- [6] Stabilisierung von Biegeträgern durch Drehbettung – eine Klarstellung, Stahlbau 56 (1987), p. 365 - 373
- [7] Stahlbau Part 2, 2nd Edition, Ulrich Krüger, Ernst & Sohn Verlag 2000
- [8] Vogel; Heil: Traglasttabellen, 4th Edition 1996, Verlag Stahleisen GmbH, Düsseldorf